Fifth SI Edition

CHAPTER

4

MECHANICS OF MATERIALS

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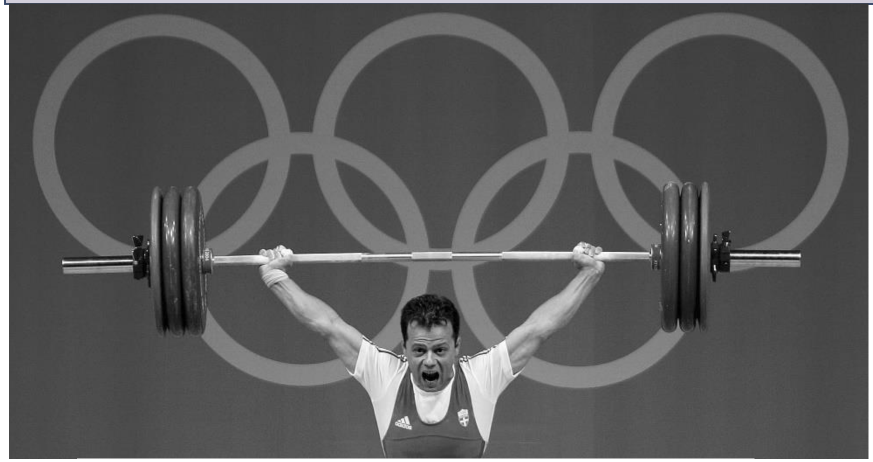
Pure Bending

Pure Bending



Pure Bending: Prismatic members subjected to equal and opposite couples acting in the same longitudinal plane

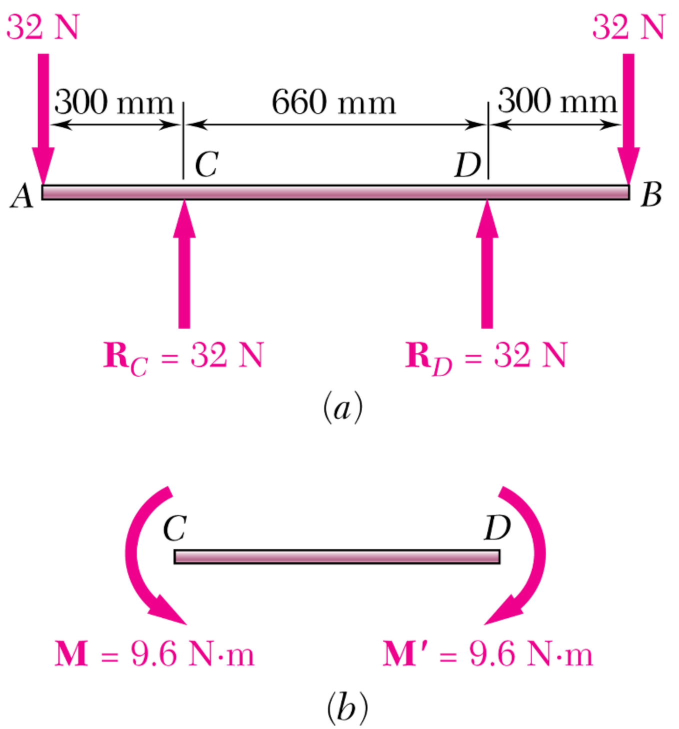
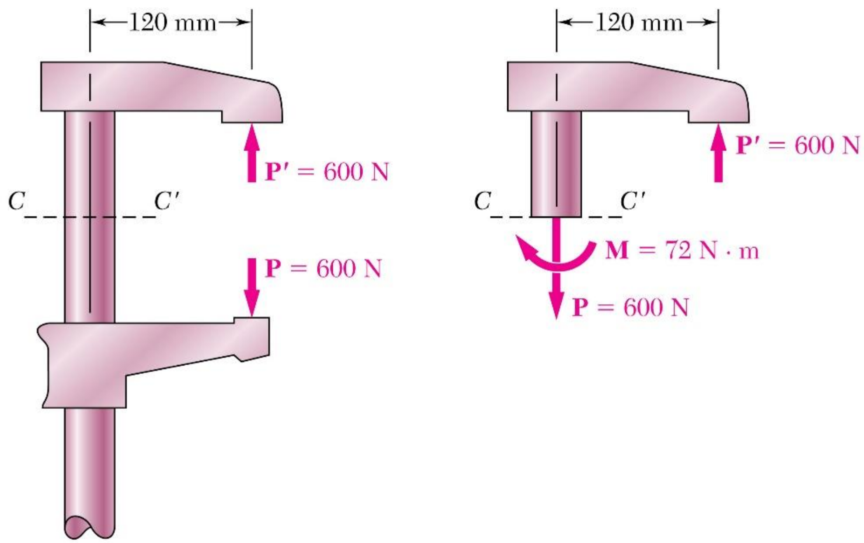
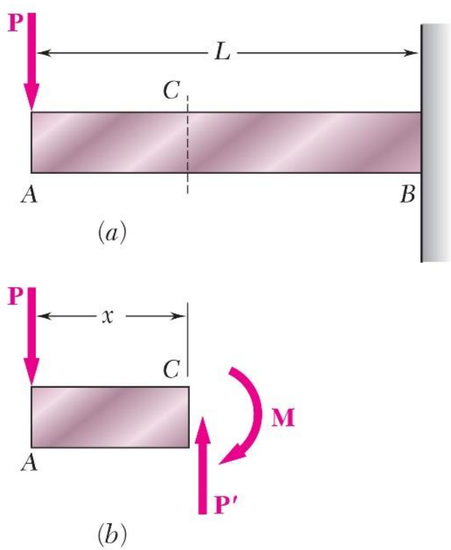


Photo 4.1 The center portion of the rear axle of the sport buggy is in pure bending.

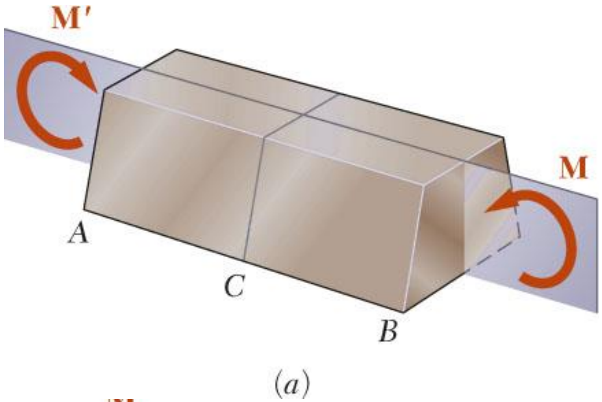
Other Loading Types



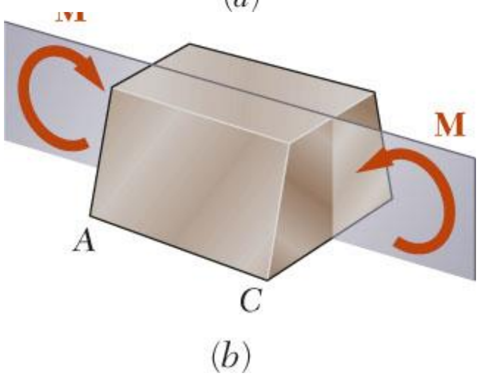


- *Eccentric Loading*: Axial loading which does not pass through section centroid produces internal forces equivalent to an axial force and a couple
- *Transverse Loading*: Concentrated or distributed transverse load produces internal forces equivalent to a shear force and a couple
- *Principle of Superposition*: The normal stress due to pure bending may be combined with the normal stress due to axial loading and shear stress due to shear loading to find the complete state of stress.

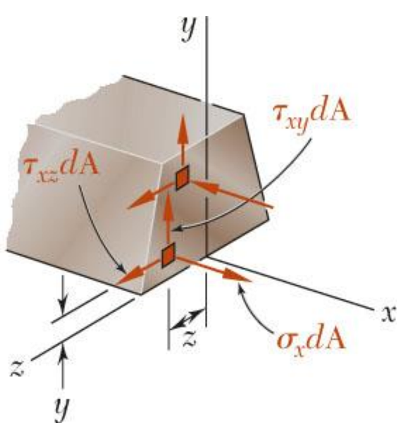
4.1 Symmetric Member in Pure Bending p.240



(a)



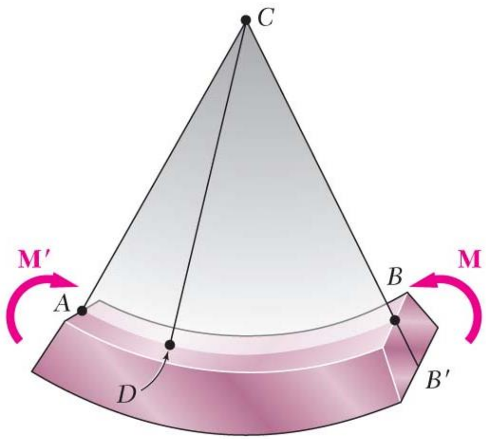
(b)

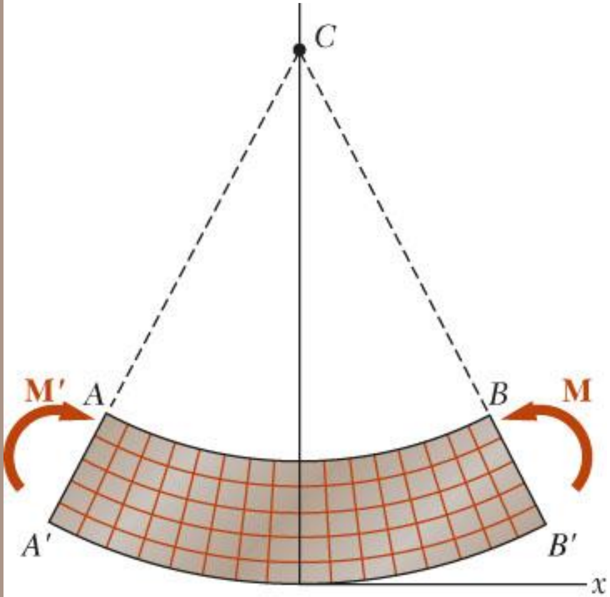


- Internal forces in any cross section are equivalent to a couple. The moment of the couple is the section *bending moment*.
- From statics, a couple M consists of two equal and opposite forces.
- The sum of the components of the forces in any direction is zero.
- The moment is the same about any axis perpendicular to the plane of the couple and zero about any axis contained in the plane.
- These requirements may be applied to the sums of the components and moments of the statically indeterminate elementary internal forces.

$$F_x = \int \sigma_x \, dA = 0$$
$$M_y = \int z\sigma_x \, dA = 0$$
$$M_z = \int -y\sigma_x \, dA = M$$

4.1B Bending Deformations



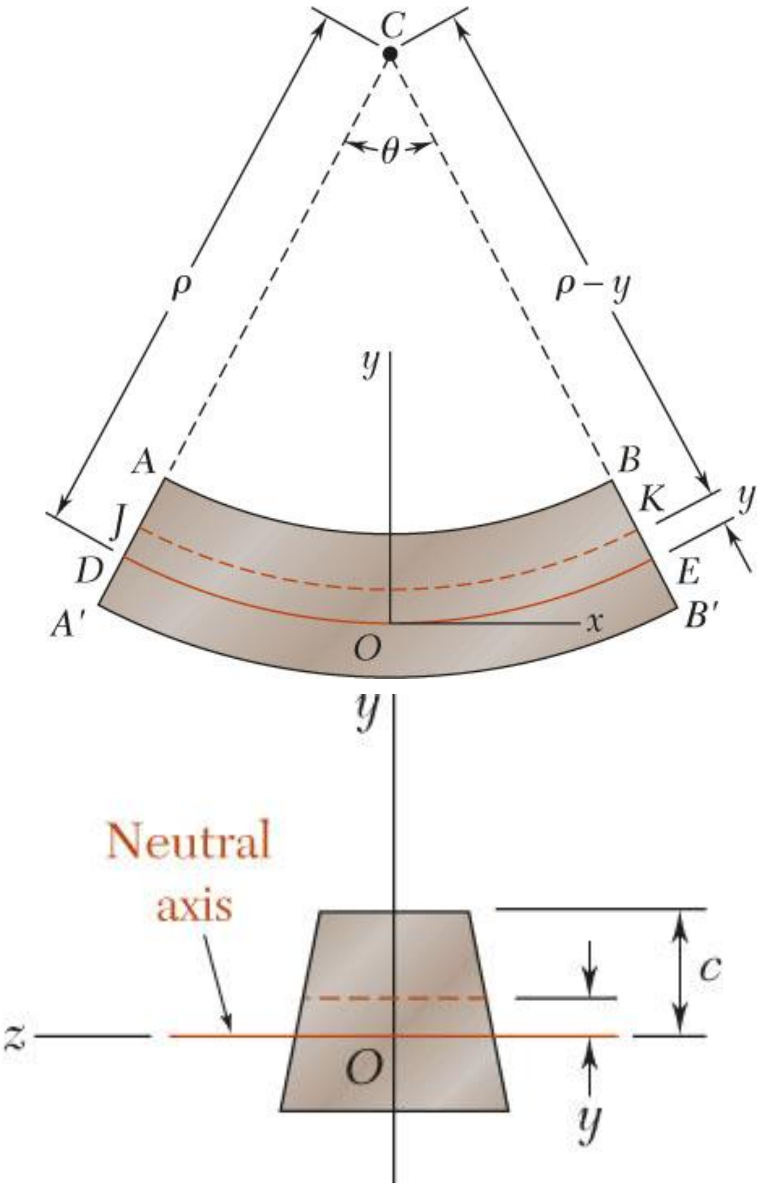


(*a*) Longitudinal, vertical section (plane of symmetry)

Beam with a plane of symmetry in pure bending:

- member remains symmetric
- bends uniformly to form a circular arc
- cross-sectional plane passes through arc center and remains planar
- length of top (AB) decreases and length of bottom (A'B') increases
- a *neutral surface* must exist that is parallel to the upper and lower surfaces and for which the length does not change ($\varepsilon_x = \sigma_x = 0$)
- stresses and strains are negative (compressive) above the neutral plane and positive (tension) below it

Strain Due to Bending



Consider a beam segment of length L.

After deformation, the length of the neutral surface remains L. At other sections,

$$L' = (\rho - y)\theta$$

$$\delta = L' - L = (\rho - y)\theta - \rho\theta = -y\theta$$

$$\varepsilon_x = \frac{\delta}{L} = -\frac{y\theta}{\rho\theta} = -\frac{y}{\rho} \quad \text{(strain varies linearly)}$$

$$\varepsilon_m = \frac{c}{\rho} \quad \text{or} \quad \rho = \frac{c}{\varepsilon_m}$$

$$\varepsilon_x = -\frac{y}{c}\varepsilon_m$$

4.2 Stress and Deformations in the Elastic Range

- For a linearly elastic material,

$$\sigma_x = E\varepsilon_x = -\frac{y}{c}E\varepsilon_m$$

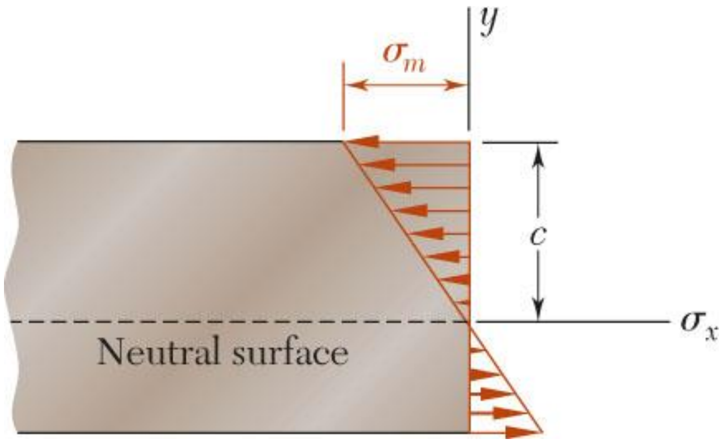
$$= -\frac{y}{c}\sigma_m \quad \text{(stress varies linearly)}$$

- For static equilibrium,

$$F_x = 0 = \int \sigma_x \, dA = \int -\frac{y}{c}\sigma_m \, dA$$

$$0 = -\frac{\sigma_m}{c}\int y \, dA$$

First moment with respect to neutral plane is zero. Therefore, the neutral surface must pass through the section centroid.



- For static equilibrium,

$$M = \int(-y\sigma_x \, dA) = \int(-y)\left(-\frac{y}{c}\sigma_m\right) dA$$

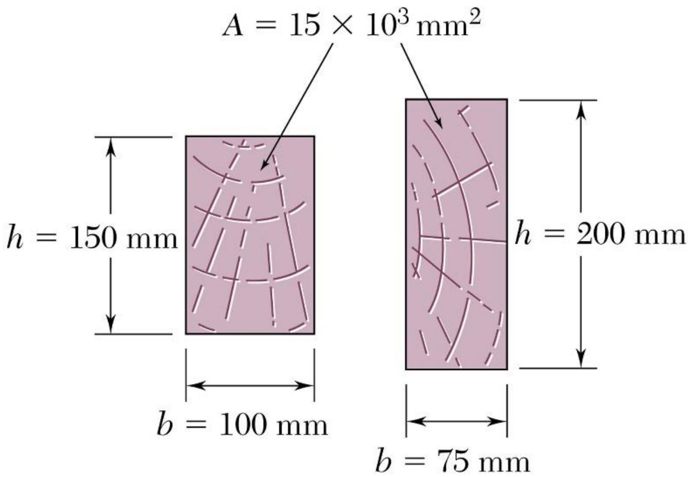
$$M = \frac{\sigma_m}{c}\int y^2 \, dA = \frac{\sigma_m I}{c}$$

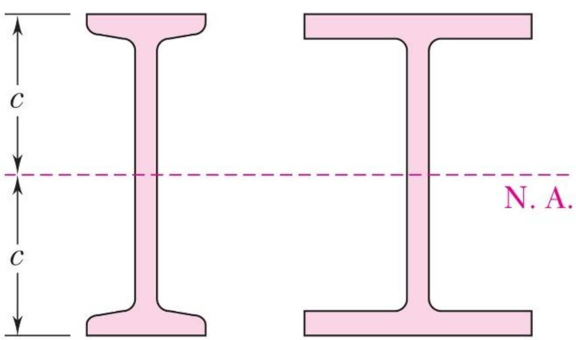
$$\sigma_m = \frac{Mc}{I} = \frac{M}{S}$$

Substituting $\sigma_x = -\frac{y}{c}\sigma_m$

$$\sigma_x = -\frac{My}{I}$$

Beam Section Properties





(a) S-beam (b) W-beam

- The maximum normal stress due to bending,

$$\sigma_m = \frac{Mc}{I} = \frac{M}{S}$$

$I =$ section moment of inertia

$$S = \frac{I}{c} = \text{section modulus}$$

A beam section with a larger section modulus will have a lower maximum stress

- Consider a rectangular beam cross section,

$$S = \frac{I}{c} = \frac{\frac{1}{12}bh^3}{h/2} = \frac{1}{6}bh^3 = \frac{1}{6}Ah$$

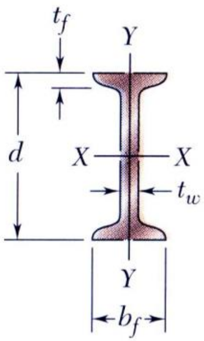
Between two beams with the same cross sectional area, the beam with the greater depth will be more effective in resisting bending.

- Structural steel beams are designed to have a large section modulus.

Properties of American Standard Shapes

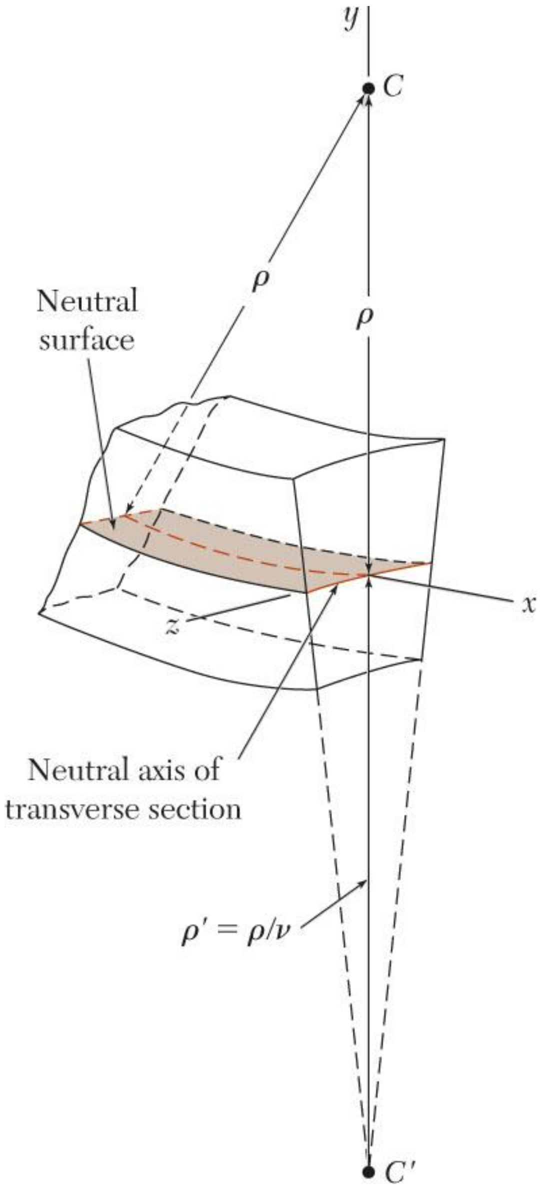
Appendix C. Properties of Rolled-Steel Shapes (SI Units)

S Shapes
(American Standard Shapes)



Designation†	Area A, mm²	Depth d, mm	Flange Width b_f, mm	Flange Thickness t_f, mm	Web Thickness t_w, mm	Axis X-X I_x 10^6 mm⁴	Axis X-X S_x 10^3 mm³	Axis X-X r_x mm	Axis Y-Y I_y 10^6 mm⁴	Axis Y-Y S_y 10^3 mm³	Axis Y-Y r_y mm
S610 × 180	22900	622	204	27.7	20.3	1320	4240	240	34.9	341	39.0
158	20100	622	200	27.7	15.7	1230	3950	247	32.5	321	39.9
149	19000	610	184	22.1	18.9	995	3260	229	20.2	215	32.3
134	17100	610	181	22.1	15.9	938	3080	234	19.0	206	33.0
119	15200	610	178	22.1	12.7	878	2880	240	17.9	198	34.0
S510 × 143	18200	516	183	23.4	20.3	700	2710	196	21.3	228	33.9
128	16400	516	179	23.4	16.8	658	2550	200	19.7	216	34.4
112	14200	508	162	20.2	16.1	530	2090	193	12.6	152	29.5
98.3	12500	508	159	20.2	12.8	495	1950	199	11.8	145	30.4
S460 × 104	13300	457	159	17.6	18.1	385	1685	170	10.4	127	27.5
81.4	10400	457	152	17.6	11.7	333	1460	179	8.83	113	28.8
S380 × 74	9500	381	143	15.6	14.0	201	1060	145	6.65	90.8	26.1
64	8150	381	140	15.8	10.4	185	971	151	6.15	85.7	27.1

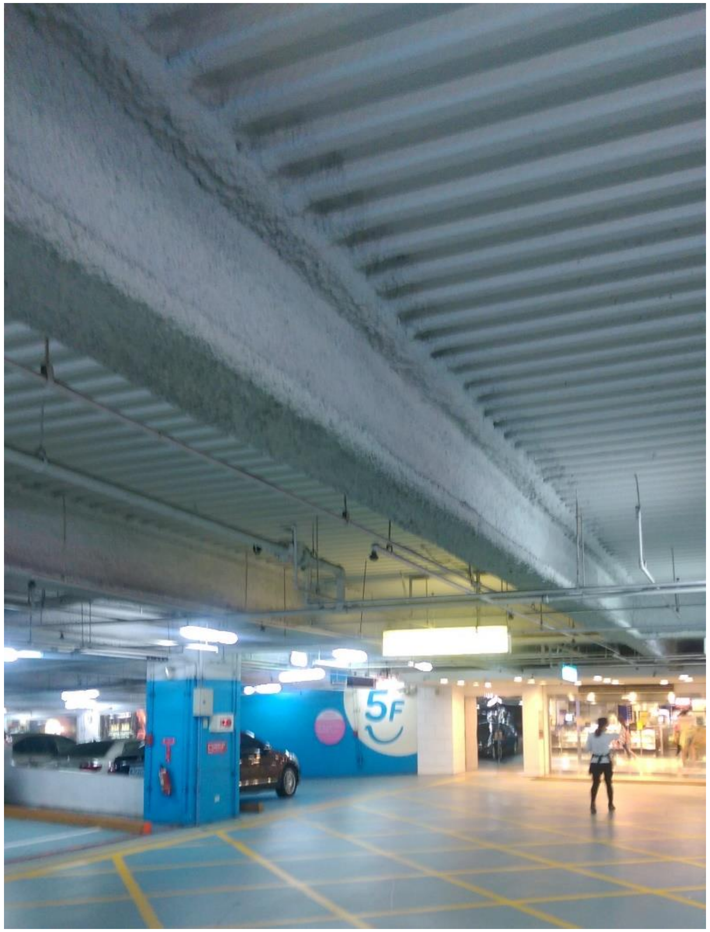
Deformations in a Transverse Cross Section



- Deformation due to bending moment M is quantified by the curvature of the neutral surface

$$\frac{1}{\rho} = \frac{\varepsilon_m}{c} = \frac{\sigma_m}{Ec} = \frac{1}{Ec}\frac{Mc}{I}$$

$$= \frac{M}{EI}$$



Concept Application 4.1

Fig. 4.17

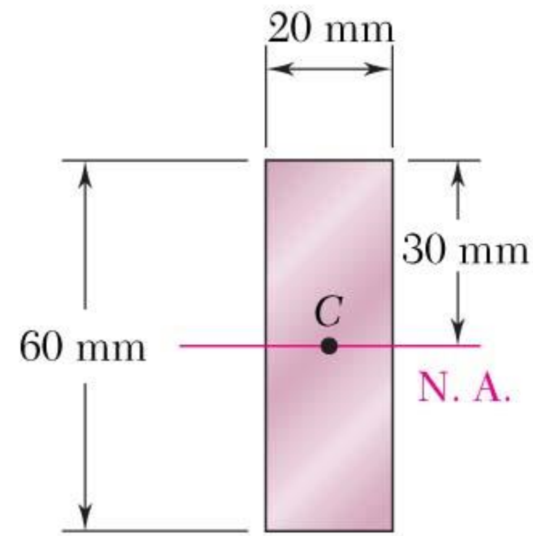


Fig. 4.18

Concept Application 4.2

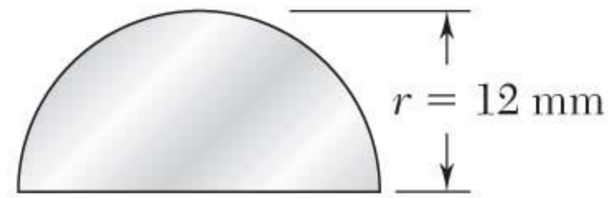


Fig. 4.19

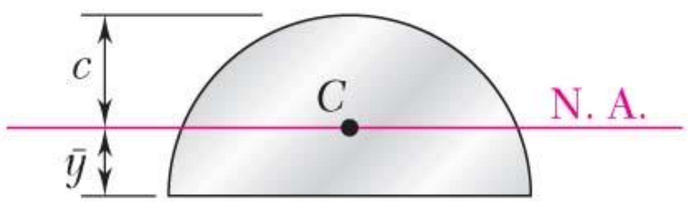
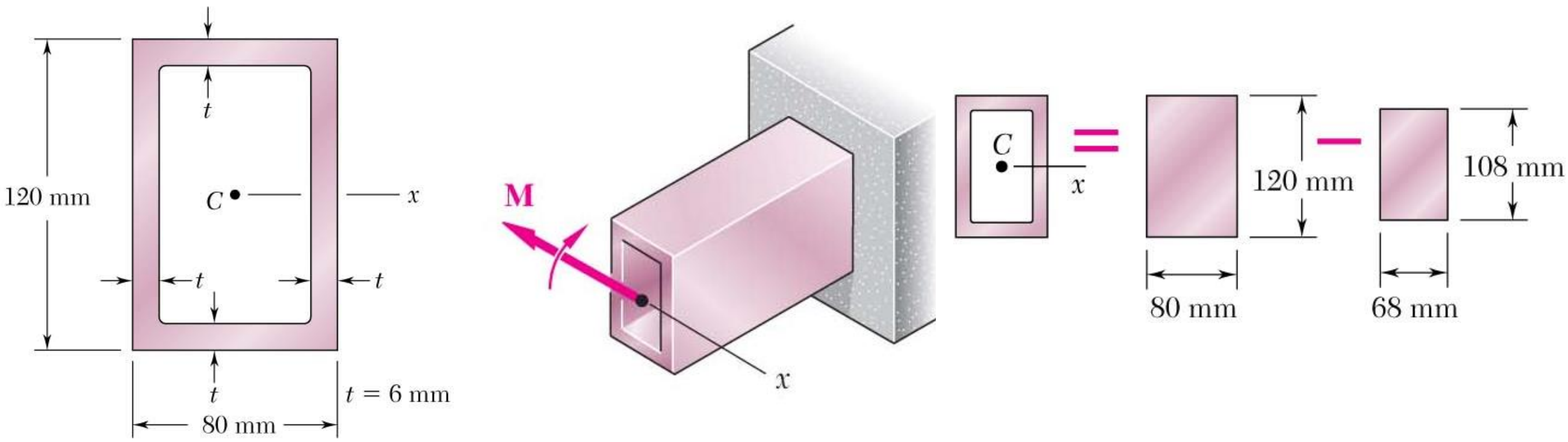
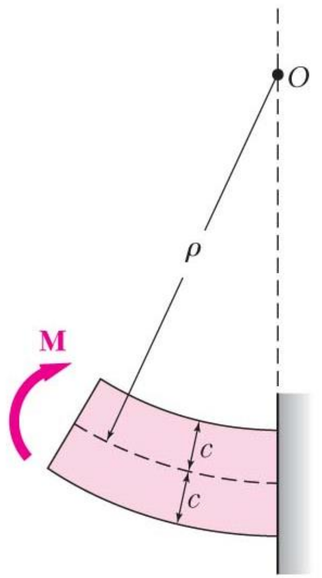


Fig. 4.20

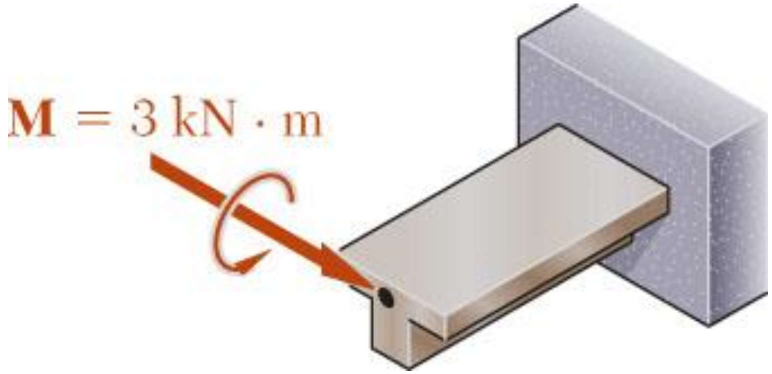
Sample Problem 4.1

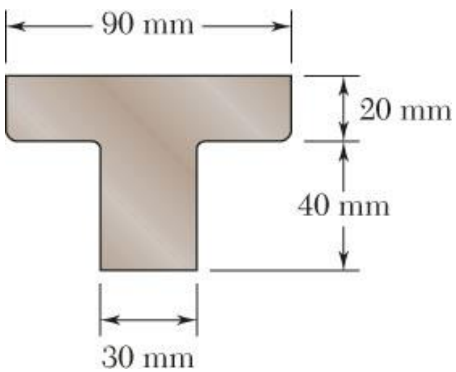


Sample Problem 4.1



Sample Problem 4.2





A cast-iron machine part is acted upon by a 3 kN-m couple. Knowing E = 165 GPa and neglecting the effects of fillets, determine (a) the maximum tensile and compressive stresses, (b) the radius of curvature.

SOLUTION:

- Based on the cross section geometry, calculate the location of the section centroid and moment of inertia.

$$\bar{Y} = \frac{\sum \bar{y}A}{\sum A} \qquad I_{x'} = \sum\left(\bar{I} + Ad^2\right)$$

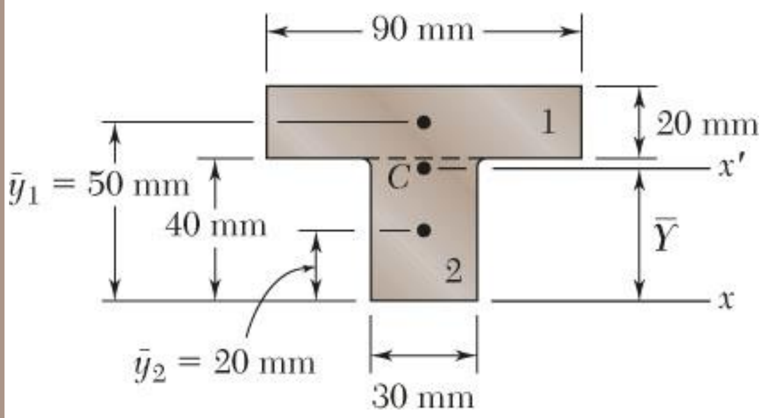
- Apply the elastic flexural formula to find the maximum tensile and compressive stresses.

$$\sigma_m = \frac{Mc}{I}$$

- Calculate the curvature

$$\frac{1}{\rho} = \frac{M}{EI}$$

Sample Problem 4.2

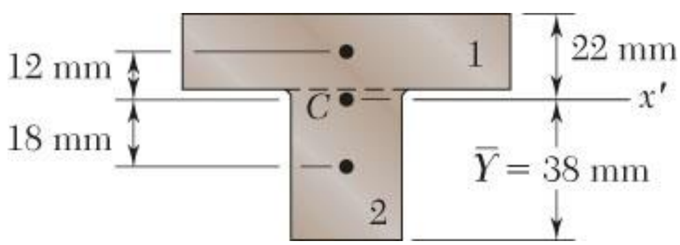


SOLUTION:

Based on the cross section geometry, calculate the location of the section centroid and moment of inertia.

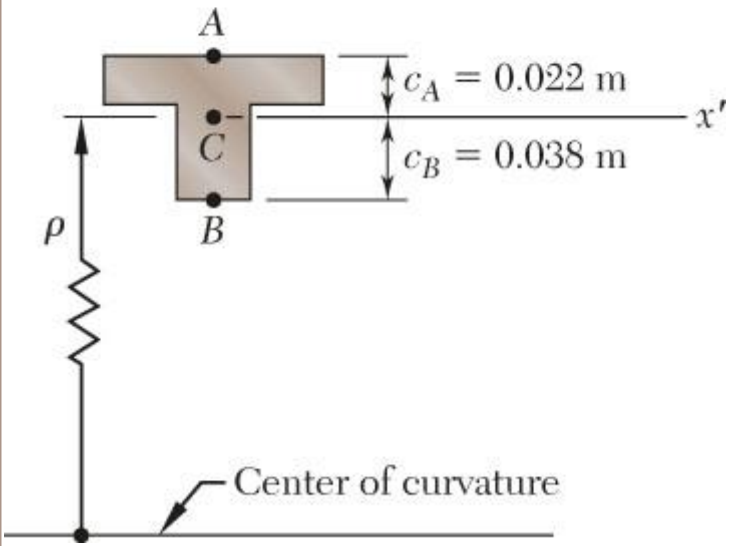
	Area, mm^2	$\bar{y}$, mm	$\bar{y}A$, mm^3
1	$20\times 90 = 1800$	50	90×10^3
2	$40\times 30 = 1200$	20	24×10^3
	$\sum A = 3000$		$\sum \bar{y}A = 114\times 10^3$

$$\bar{Y} = \frac{\sum \bar{y}A}{\sum A} = \frac{114\times 10^3}{3000} = 38\,\text{mm}$$



$$I_{x'} = \sum\left(\bar{I} + Ad^2\right) = \sum\left(\tfrac{1}{12}bh^3 + Ad^2\right)$$
$$= \left(\tfrac{1}{12}90\times 20^3 + 1800\times 12^2\right) + \left(\tfrac{1}{12}30\times 40^3 + 1200\times 18^2\right)$$
$$I = 868\times 10^3\,\text{mm}^4 = 868\times 10^{-9}\,\text{m}^4$$

Sample Problem 4.2



- Apply the elastic flexural formula to find the maximum tensile and compressive stresses.

$$\sigma_m = \frac{Mc}{I}$$

$$\sigma_A = \frac{M\,c_A}{I} = \frac{3\,\mathrm{kN\cdot m} \times 0.022\,\mathrm{m}}{868\times10^{-9}\,\mathrm{m}^4}$$

$$\boxed{\sigma_A = +76.0\ \mathrm{MPa}}$$

$$\sigma_B = -\frac{M\,c_B}{I} = -\frac{3\,\mathrm{kN\cdot m} \times 0.038\,\mathrm{m}}{868\times10^{-9}\,\mathrm{m}^4}$$

$$\boxed{\sigma_B = -131.3\ \mathrm{MPa}}$$

- Calculate the curvature

$$\frac{1}{\rho} = \frac{M}{EI}$$

$$= \frac{3\,\mathrm{kN\cdot m}}{(165\,\mathrm{GPa})(868\times10^{-9}\,\mathrm{m}^4)}$$

$$\boxed{\frac{1}{\rho} = 20.95\times10^{-3}\,\mathrm{m}^{-1}}$$

$$\boxed{\rho = 47.7\ \mathrm{m}}$$

Problems

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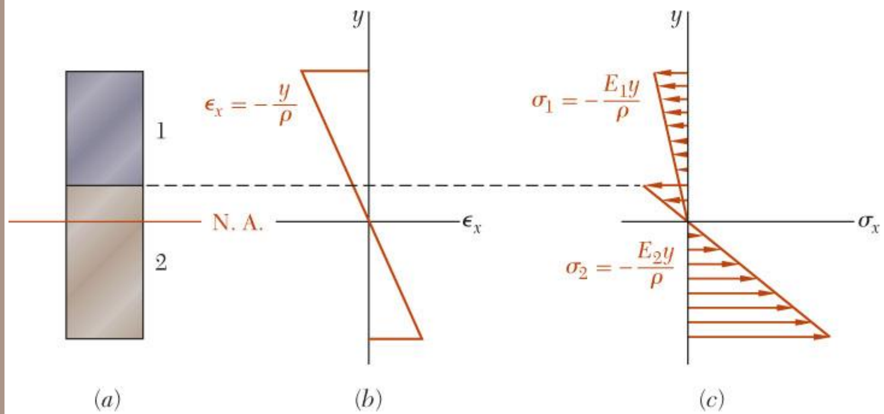
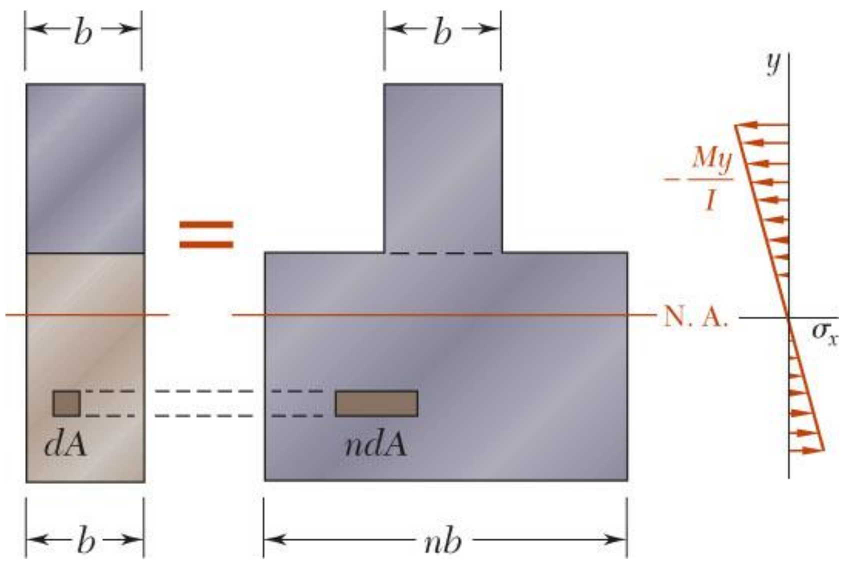
4.4 Members Made of Composite Materials p259





$$\sigma_x = -\frac{My}{I}$$

$$\sigma_1 = \sigma_x \qquad \sigma_2 = n\sigma_x$$

- Consider a composite beam formed from two materials with E_1 and E_2.
- Normal strain varies linearly.

$$\varepsilon_x = -\frac{y}{\rho}$$

- Piecewise linear normal stress variation.

$$\sigma_1 = E_1\varepsilon_x = -\frac{E_1 y}{\rho} \qquad \sigma_2 = E_2\varepsilon_x = -\frac{E_2 y}{\rho}$$

Neutral axis does not pass through section centroid of composite section.

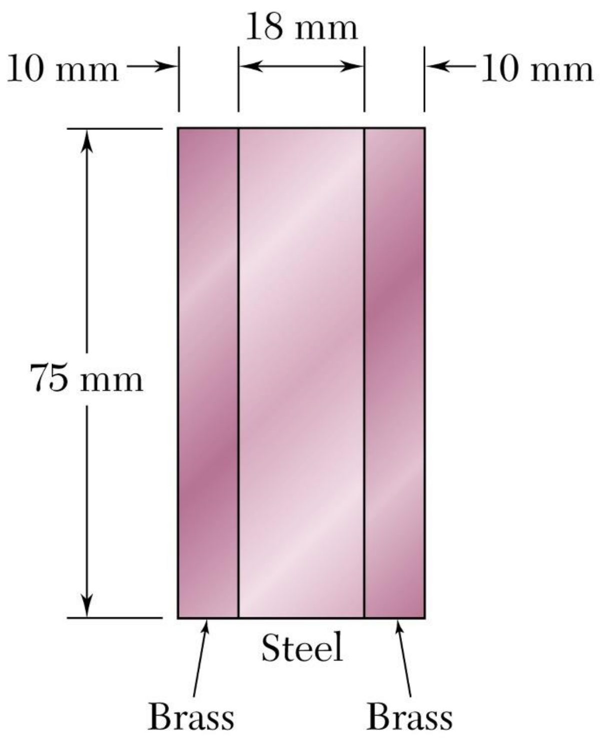
- Elemental forces on the section are

$$dF_1 = \sigma_1 dA = -\frac{E_1 y}{\rho} dA \quad dF_2 = \sigma_2 dA = -\frac{E_2 y}{\rho} dA$$

- Define a transformed section such that

$$dF_2 = -\frac{(nE_1)y}{\rho} dA = -\frac{E_1 y}{\rho}(n\,dA) \qquad n = \frac{E_2}{E_1}$$

Concept Application 4.3

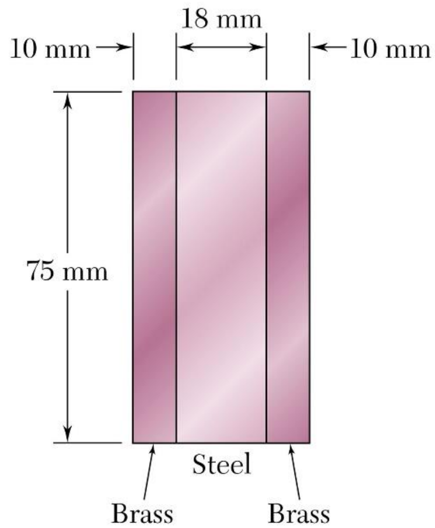


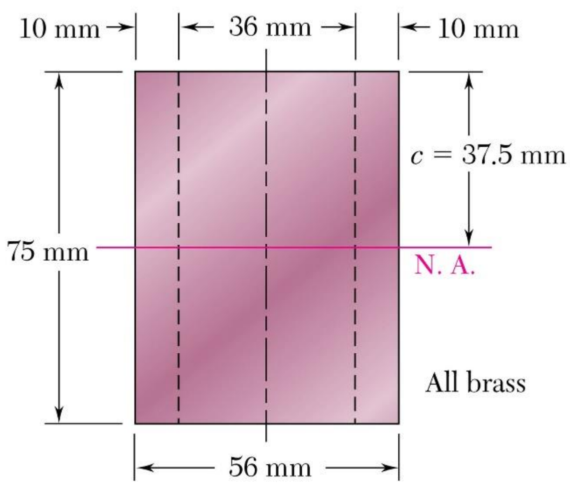
Bar is made from bonded pieces of steel (E_s = 200 GPa) and brass (E_b = 100 GPa). Determine the maximum stress in the steel and brass when a moment of 4.5 KNm is applied.

SOLUTION:

- Transform the bar to an equivalent cross section made entirely of brass
- Evaluate the cross sectional properties of the transformed section
- Calculate the maximum stress in the transformed section. This is the correct maximum stress for the brass pieces of the bar.
- Determine the maximum stress in the steel portion of the bar by multiplying the maximum stress for the transformed section by the ratio of the moduli of elasticity.

Concept Application 4.3





SOLUTION:

- Transform the bar to an equivalent cross section made entirely of brass.

$$n = \frac{E_s}{E_b} = \frac{200\text{GPa}}{100\text{GPa}} = 2.0$$

$$b_T = 10\text{ mm} + 2\times 18\text{ mm} + 10\text{ mm} = 56\text{ mm}$$

- Evaluate the transformed cross sectional properties

$$I = \tfrac{1}{12} b_T h^3 = \tfrac{1}{12}(56\text{ mm})(75\text{ mm})^3$$
$$= 1.96875\times 10^6\text{ mm}^4$$

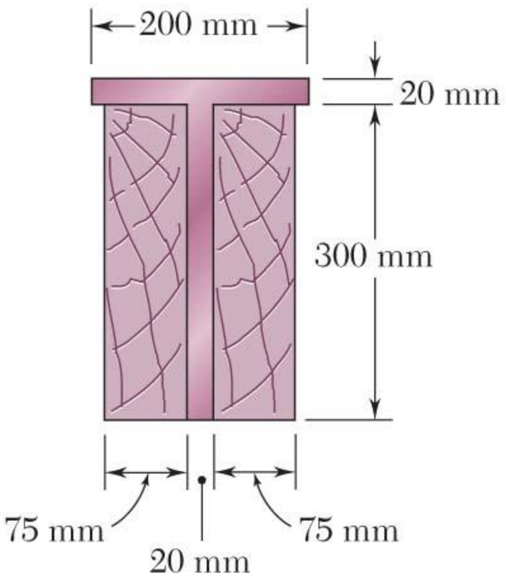
- Calculate the maximum stresses

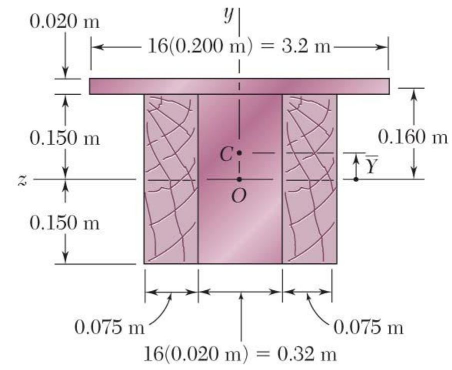
$$\sigma_m = \frac{Mc}{I} = \frac{(4500\text{ Nm})(0.0375\text{ m})}{1.96875\times 10^{-6}\text{ m}^4} = 85.7\text{ MPa}$$

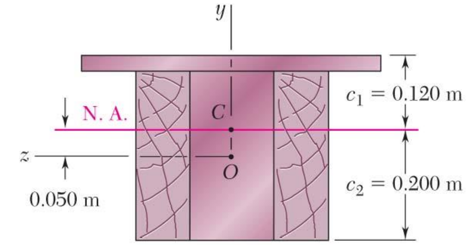
$$(\sigma_b)_{\max} = \sigma_m$$
$$(\sigma_s)_{\max} = n\sigma_m = 2\times 85.7\text{ MPa}$$

$$(\sigma_b)_{\max} = 85.7\text{ MPa}$$
$$(\sigma_s)_{\max} = 171.4\text{ MPa}$$

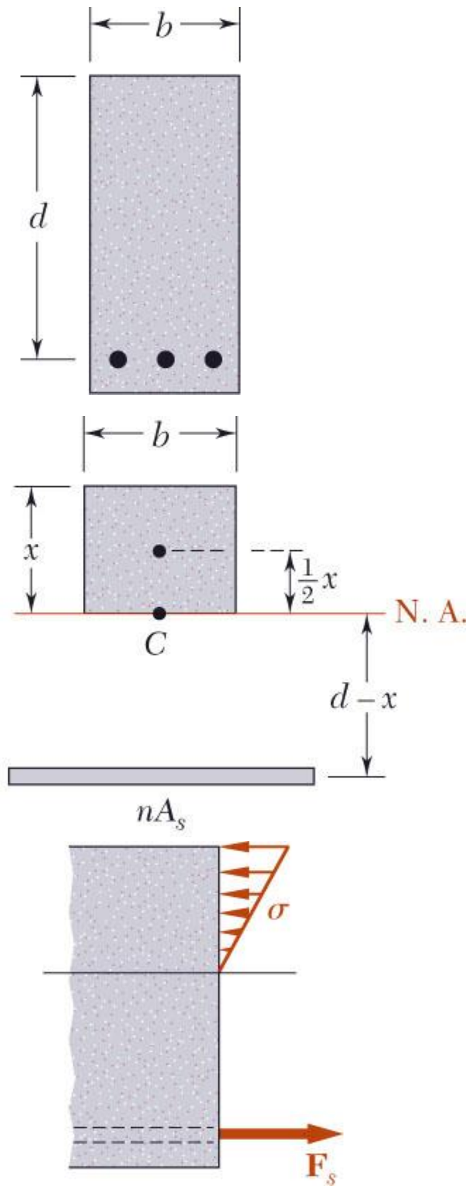
Sample Problem 4.3







Reinforced Concrete Beams



- Concrete beams subjected to bending moments are reinforced by steel rods.
- The steel rods carry the entire tensile load below the neutral surface. The upper part of the concrete beam carries the compressive load.
- In the transformed section, the cross sectional area of the steel, A_s, is replaced by the equivalent area nA_s where $n = E_s/E_c$.
- To determine the location of the neutral axis,

$$(bx)\frac{x}{2} - nA_s(d - x) = 0$$

$$\tfrac{1}{2}bx^2 + nA_s x - nA_s d = 0$$

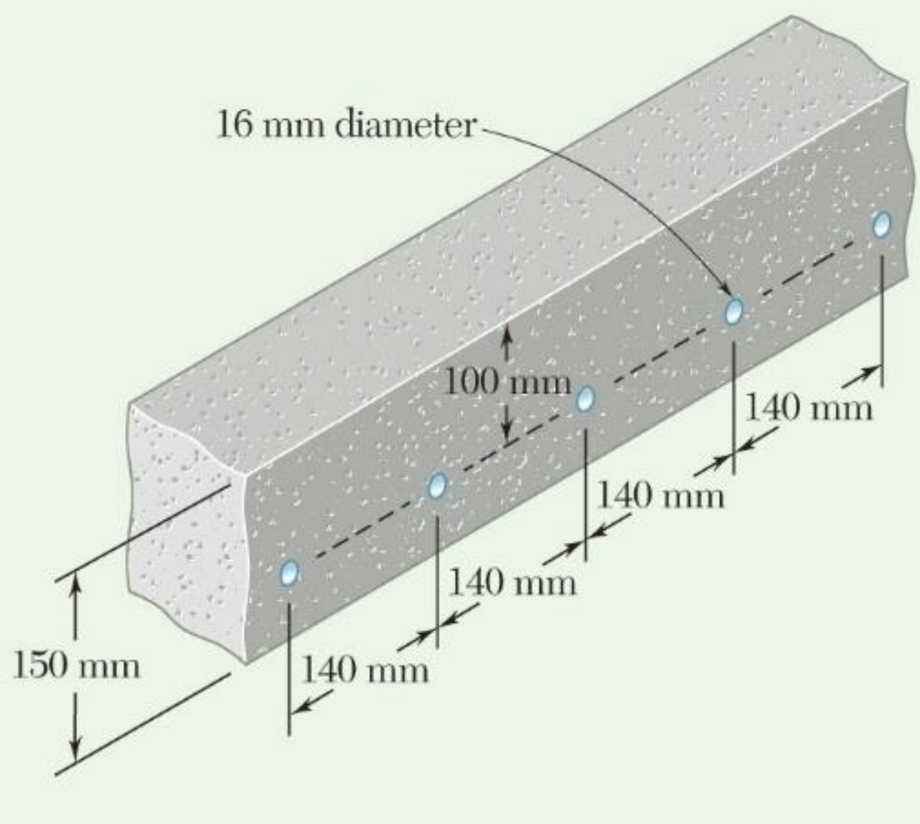
- The normal stress in the concrete and steel

$$\sigma_x = -\frac{My}{I}$$

$$\sigma_c = \sigma_x \qquad \sigma_s = n\sigma_x$$

Sample Problem 4.4

A concrete floor slab is reinforced with 16-mm-diameter steel rods. The modulus of elasticity is 200 GPa for steel and 20 Gpa for concrete. Using an allowable stress of 9 MPa for the concrete and 140MPa for the steel, determine the largerest bending moment per meter of width that can be sfely applied to the slab.



Sample Problem 4.4

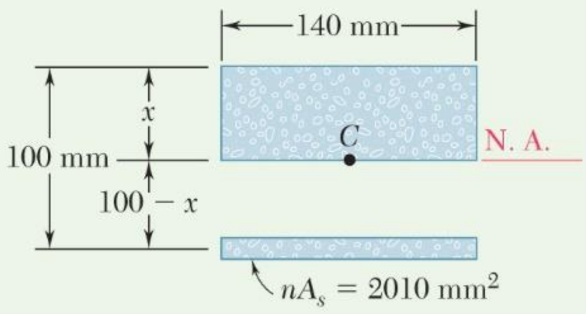


Fig. 1 Transformed section.

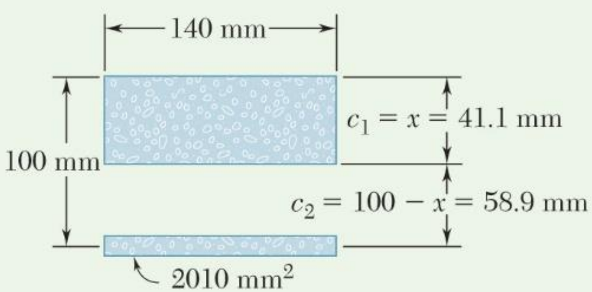


Fig. 2 Dimensions of transformed section used to calculate moment of inertia.

SOLUTION:

- Transform to a section made entirely of concrete.

$$n = \frac{E_s}{E_c} = \frac{200\,\text{GPa}}{20\,\text{GPa}} = 16.0$$

$$nA_s = 16.0 \times \left[\tfrac{\pi}{4}(16\text{mm})^2\right] = 2010\,\text{mm}^2$$

- Evaluate the geometric properties of the transformed section.

$$140x\left(\frac{x}{2}\right) - 2010(100 - x) = 0 \qquad x = 41.1\text{mm}$$

$$I = \tfrac{1}{3}(140)(41.1\text{mm})^3 + \left(2010\,\text{mm}^2\right)(58.9\,\text{mm})^2 = 10.21\times10^6\ \text{mm}^4$$

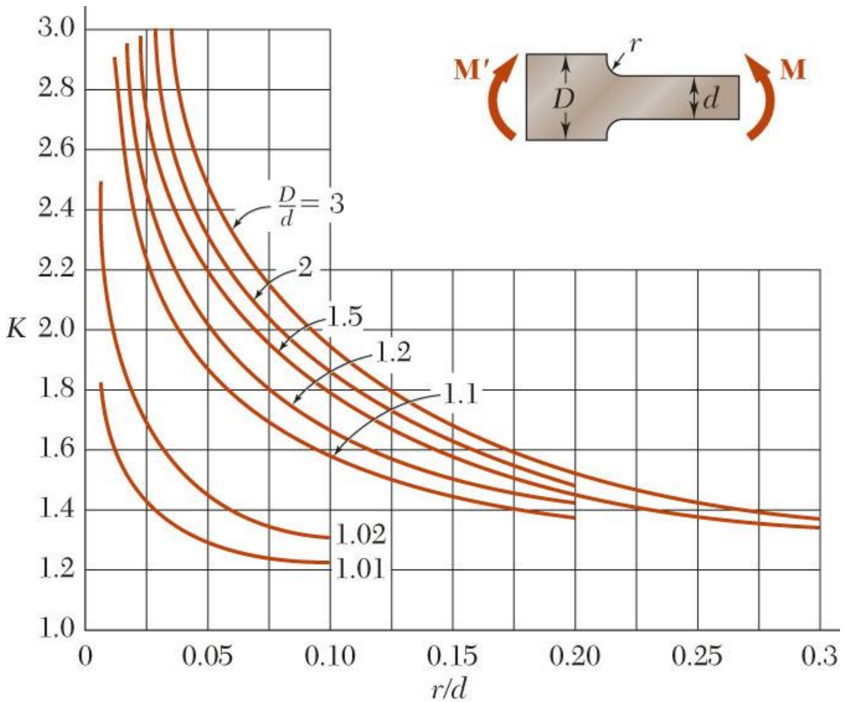
- Calculate the maximum stresses.

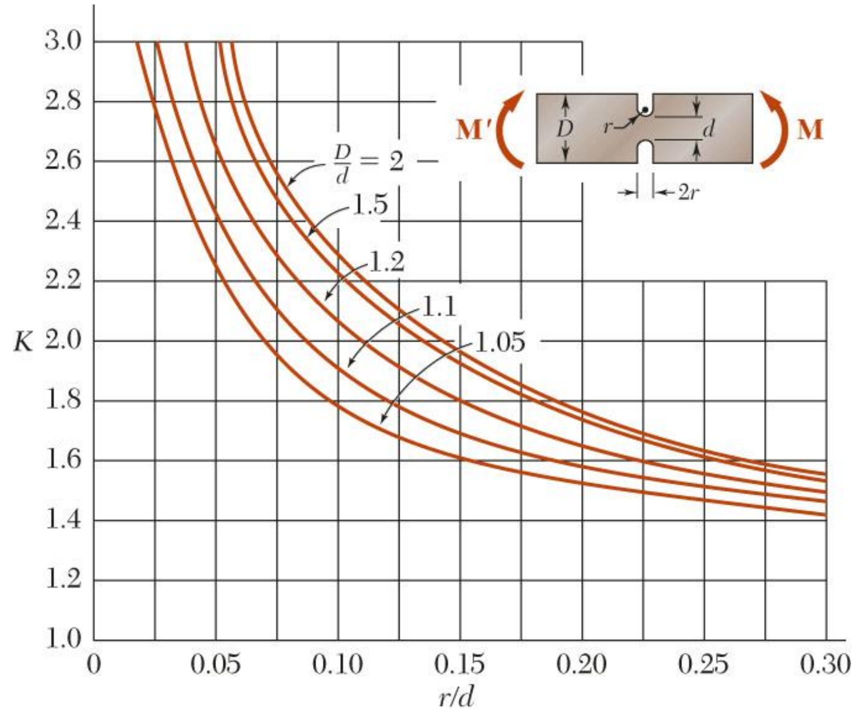
$$n = 1, y = 41.1, \sigma = 9MPa$$

$$M_c = \frac{\sigma_C I}{c_1} = \frac{10.21\times10^{-6}\times9\times10}{(1.0)(0.0411)} = 2236Nm$$

$$M_s = \frac{\sigma_s I}{nc_2} = \frac{10.21\times10^{-6}\times140\times10^6}{(10)(0.0589)} = 2428Nm$$

4.5 Stress Concentrations p.263





Stress concentrations may occur:

- in the vicinity of points where the loads are applied
- in the vicinity of abrupt changes in cross section

$$\sigma_m = K\frac{Mc}{I}$$

Concept Application 4.4

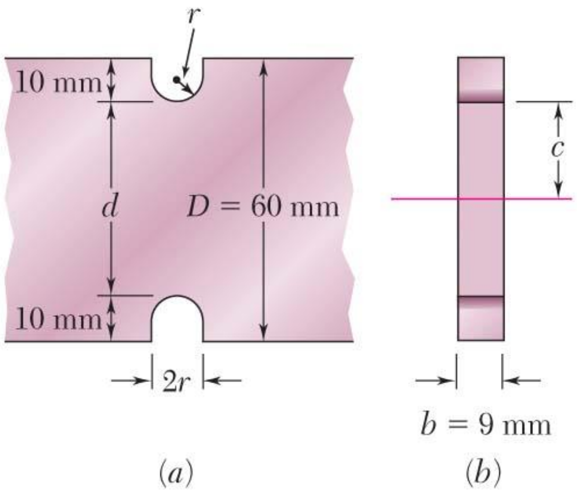


Fig. 4.33

Problems

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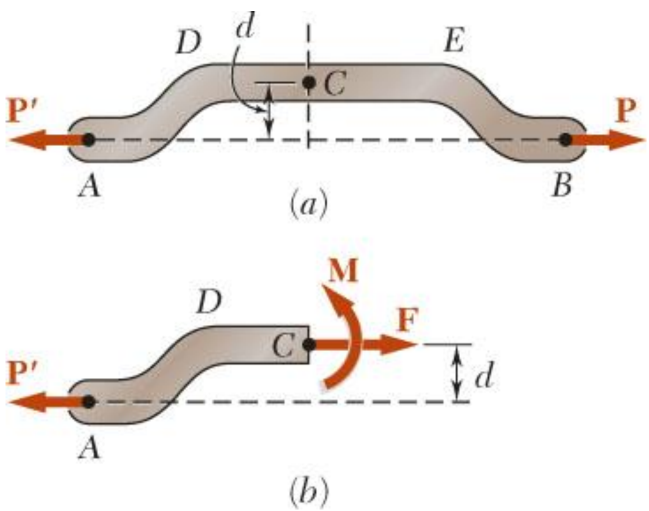
4.7 Eccentric Axial Loading in a Plane of Symmetry



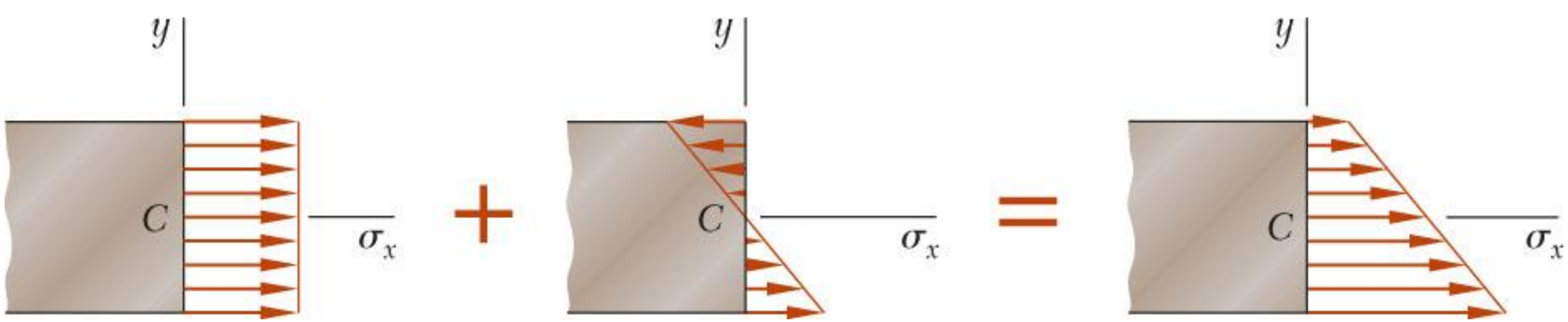
- Eccentric loading

$$F = P$$
$$M = Pd$$

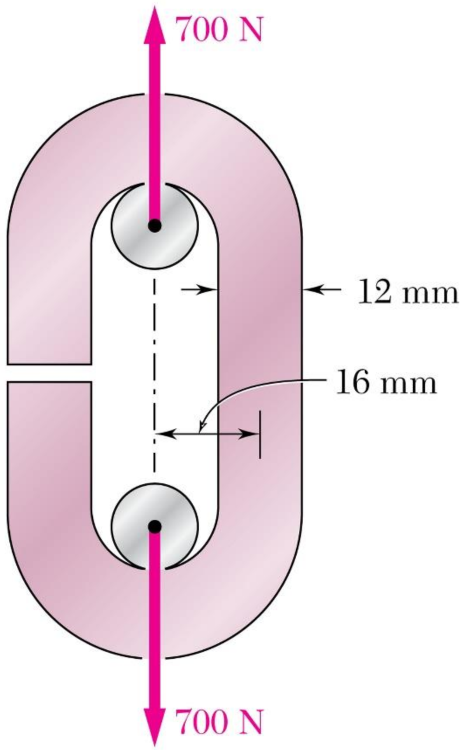
- Stress due to eccentric loading found by superposing the uniform stress due to a centric load and linear stress distribution due a pure bending moment

$$\sigma_x = (\sigma_x)_{\text{centric}} + (\sigma_x)_{\text{bending}}$$
$$= \frac{P}{A} - \frac{My}{I}$$

- Validity requires stresses below proportional limit, deformations have negligible effect on geometry, and stresses not evaluated near points of load application.



Concept Application 4.7

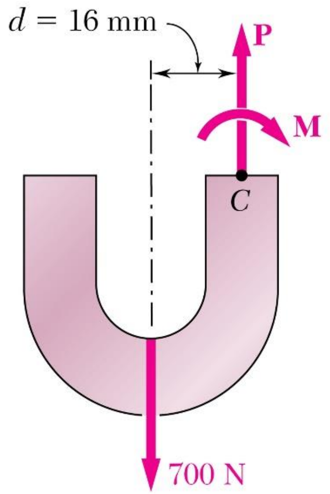


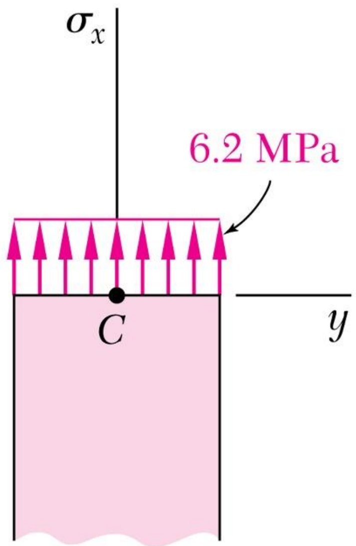
An open-link chain is obtained by bending low-carbon steel rods into the shape shown. For 700 N load, determine (a) maximum tensile and compressive stresses, (b) distance between section centroid and neutral axis

SOLUTION:

- Find the equivalent centric load and bending moment
- Superpose the uniform stress due to the centric load and the linear stress due to the bending moment.
- Evaluate the maximum tensile and compressive stresses at the inner and outer edges, respectively, of the superposed stress distribution.
- Find the neutral axis by determining the location where the normal stress is zero.

Concept Application 4.7



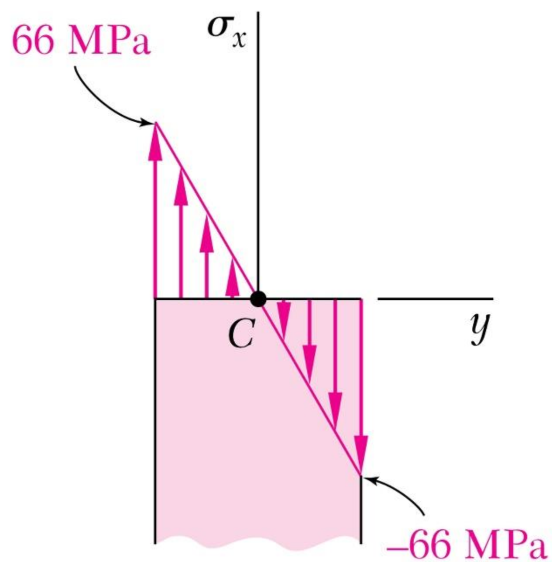


- Normal stress due to a centric load

$$A = \pi c^2 = \pi (6\,\text{mm})^2$$
$$= 113.1\,\text{mm}^2$$
$$\sigma_0 = \frac{P}{A} = \frac{700\,\text{N}}{113.1\times 10^{-6}\,\text{m}^2}$$
$$= 6.2\,\text{MPa}$$

- Equivalent centric load and bending moment

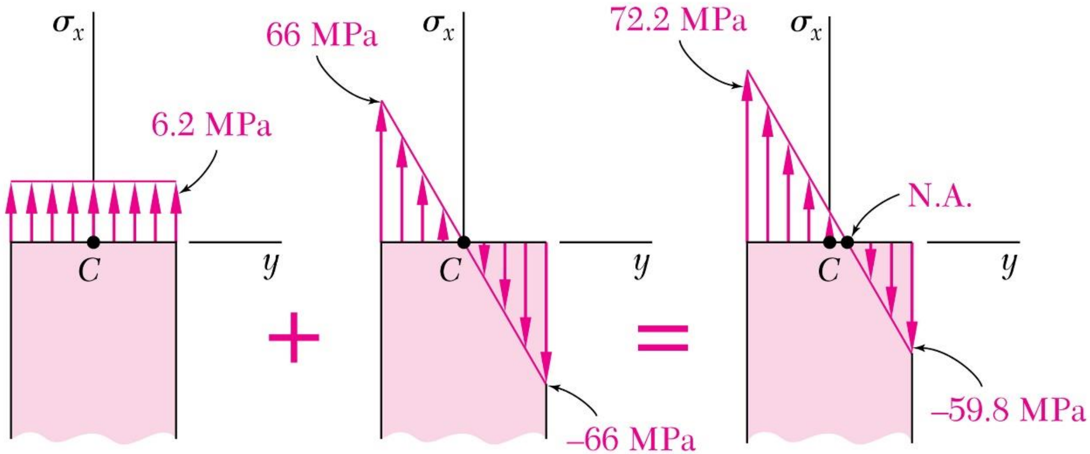
$$P = 700\,\text{N}$$
$$M = Pd = (700\,\text{N})(0.016\,\text{m})$$
$$= 11.2\,\text{Nm}$$



- Normal stress due to bending moment

$$I = \tfrac{1}{4}\pi c^4 = \tfrac{1}{4}\pi (6\,\text{mm})^4$$
$$= 1017.9\,\text{mm}^4$$
$$\sigma_m = \frac{Mc}{I} = \frac{(11.2\,\text{Nm})(0.006\,\text{m})}{1017.9\times 10^{-12}\,\text{m}^4}$$
$$= 66\,\text{MPa}$$

Concept Application 4.7



- Maximum tensile and compressive stresses

$$\sigma_t = \sigma_0 + \sigma_m = 6.2 + 66$$

$$\boxed{\sigma_t = 72.2\,\text{MPa}}$$

$$\sigma_c = \sigma_0 - \sigma_m = 6.2 - 66$$

$$\boxed{\sigma_c = -59.8\,\text{MPa}}$$

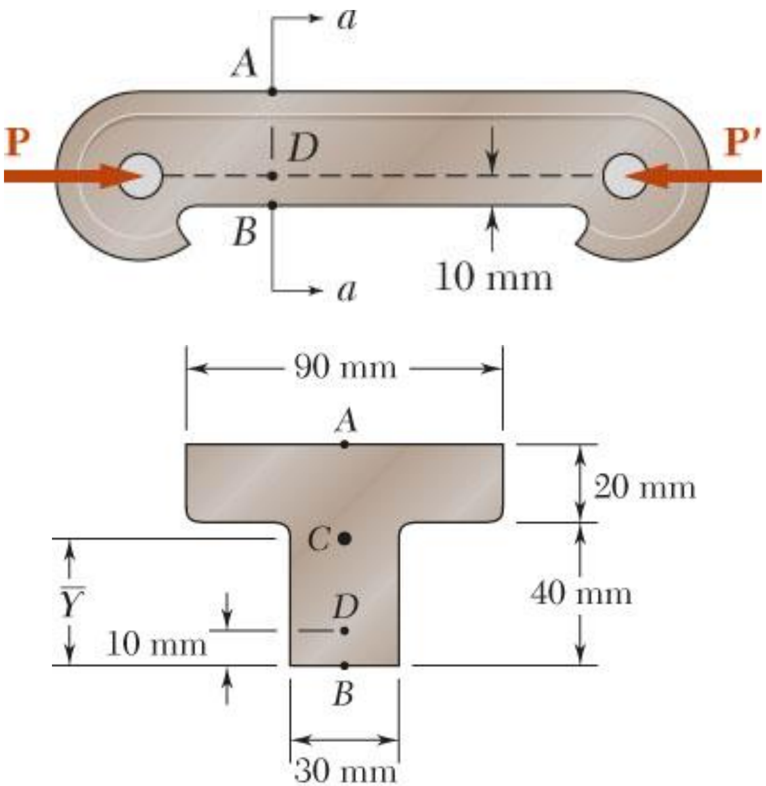
- Neutral axis location

$$0 = \frac{P}{A} - \frac{My_0}{I}$$

$$y_0 = \frac{P}{A}\frac{I}{M} = \left(6.2\times10^6\ \text{Pa}\right)\frac{1017.9\times10^{-12}\,\text{m}^4}{11.2\,\text{Nm}}$$

$$\boxed{y_0 = 0.56\,\text{mm}}$$

Sample Problem 4.8



Section a–a

The largest allowable stresses for the cast iron link are 30 MPa in tension and 120 MPa in compression. Determine the largest force *P* which can be applied to the link.

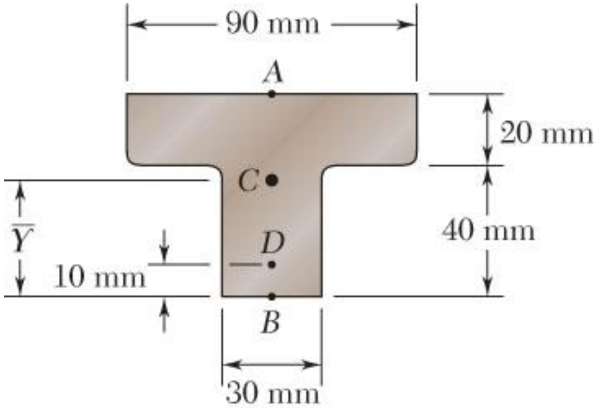
From Sample Problem 4.2,

$$A = 3\times10^{-3}\,\text{m}^2$$
$$\bar{Y} = 0.038\,\text{m}$$
$$I = 868\times10^{-9}\,\text{m}^4$$

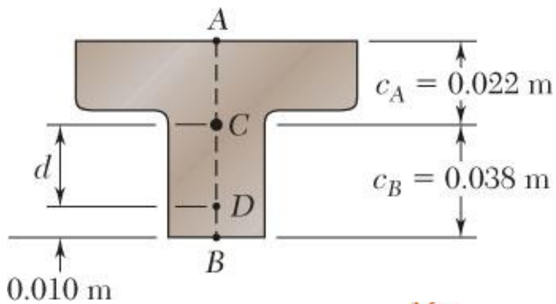
SOLUTION:

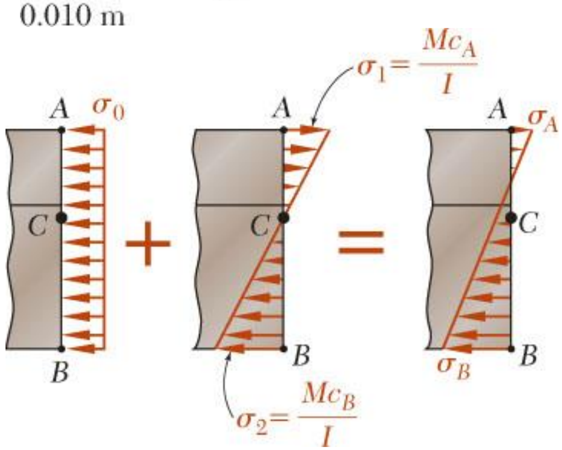
- Determine equivalent centric load and bending moment.
- Superpose the stress due to a centric load and the stress due to bending.
- Evaluate the critical loads for the allowable tensile and compressive stresses.
- The largest allowable load is the smallest of the two critical loads.

Sample Problem 4.8



Section a–a





- Determine equivalent centric and bending loads.

$$d = 0.038 - 0.010 = 0.028\,\mathrm{m}$$

$$P = \text{centric load}$$

$$M = Pd = 0.028P = \text{bending moment}$$

- Superpose stresses due to centric and bending loads

$$\sigma_A = -\frac{P}{A} + \frac{Mc_A}{I} = -\frac{P}{3\times10^{-3}} + \frac{(0.028P)(0.022)}{868\times10^{-9}} = +377P$$

$$\sigma_B = -\frac{P}{A} - \frac{Mc_A}{I} = -\frac{P}{3\times10^{-3}} - \frac{(0.028P)(0.022)}{868\times10^{-9}} = -1559P$$

- Evaluate critical loads for allowable stresses.

$$\sigma_A = +377P = 30\,\mathrm{MPa} \qquad P = 79.6\,\mathrm{kN}$$

$$\sigma_B = -1559P = -120\,\mathrm{MPa} \qquad P = 77.0\,\mathrm{kN}$$

- The largest allowable load $\boxed{P = 77.0\ \mathrm{kN}}$

Problems

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Problem 4.18

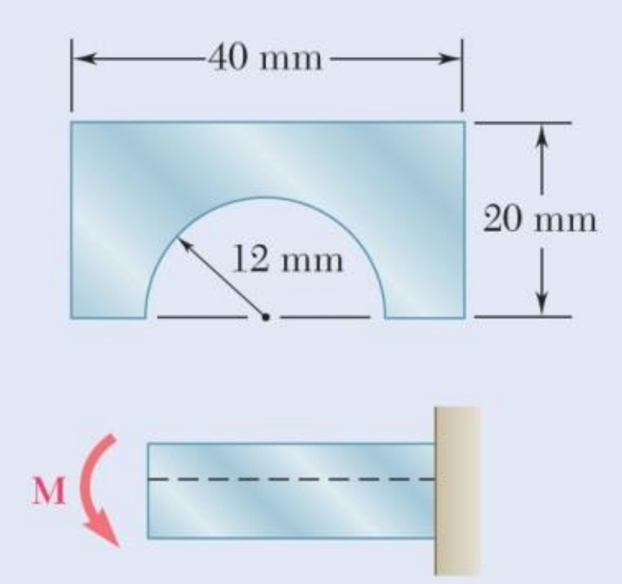


Fig. P4.18

Allowable stress 80MPa tensile

Allowable stress 100 MPa compression

Problem 4.34

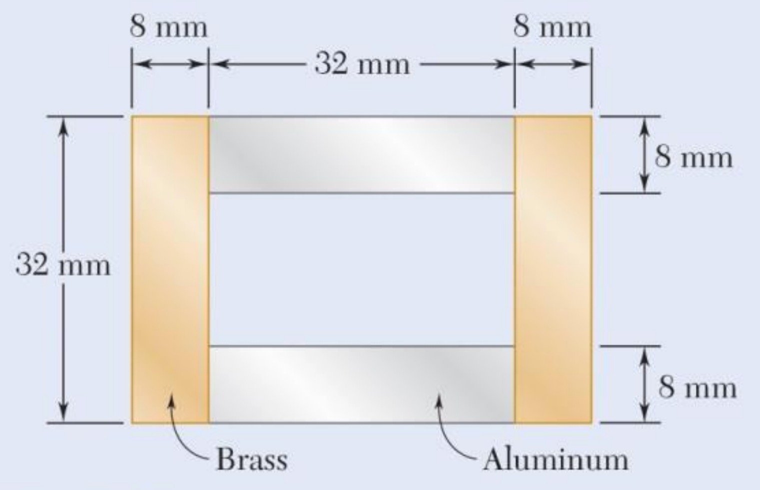
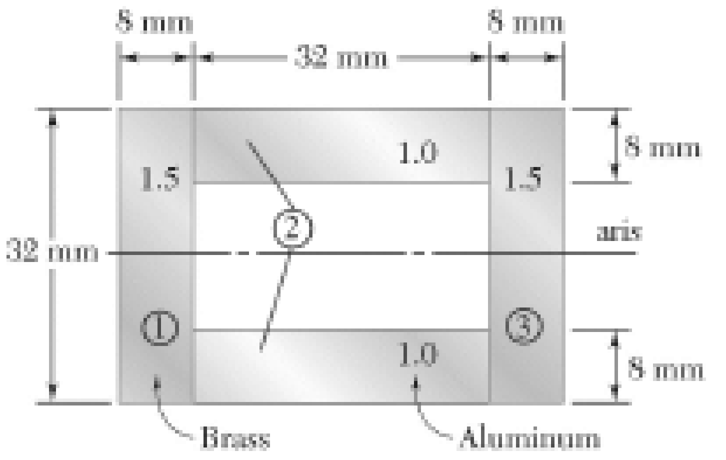


Fig. P4.34



$M = 887\ \mathrm{N \cdot m}$ ◀

Problem 4.41

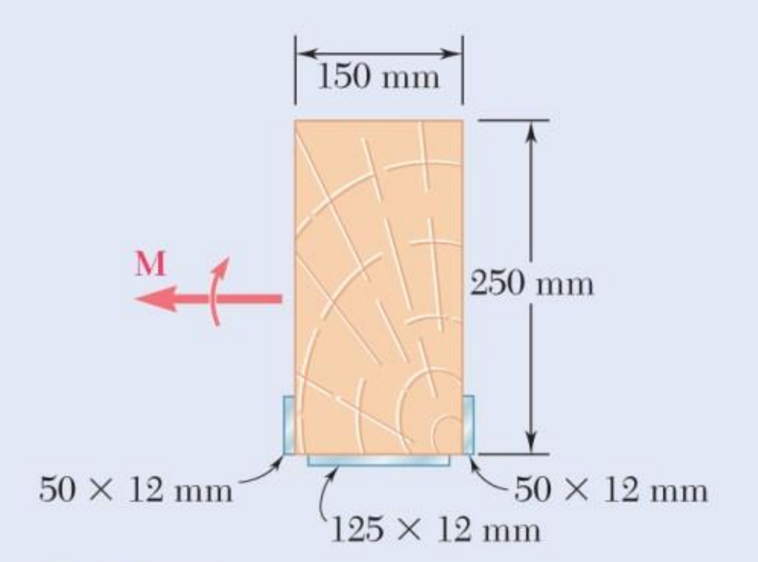
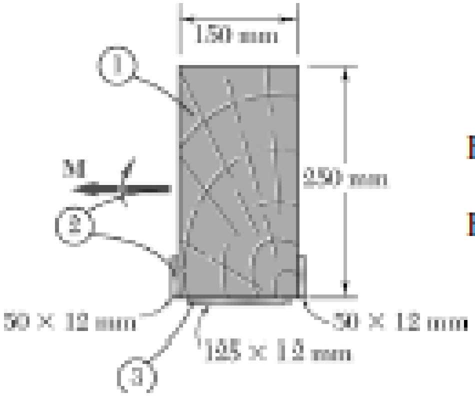


Fig. P4.41



For wood, $n = 1$

For steel, $n = \frac{E_s}{E_w} = \frac{200}{12} = 16.667$

	A, mm²	nA, mm²	$\bar{y}$, mm	$nA\bar{y}$, mm³
①	37500	37500	137	5137500
②	1200	20000.4	37	740014.8
③	1500	25000.5	6	150003
Σ		82500.9		6027517.8

$$\bar{Y} = \frac{6027517.8}{82500.9} = 73 \text{ mm}$$

Distances from neutral axis.

$$d_1 = 137 - 73 = 64 \text{ mm}$$
$$d_2 = |37 - 73| = 36 \text{ mm}$$
$$d_3 = |6 - 73| = 67 \text{ mm}$$

$$I_1 = \frac{n_1}{12} b_1 h_1^3 + n_1 A_1 d_1^2 = \frac{1}{12}(150)(250)^3 + (37500)(64)^2 = 348.91 \times 10^6 \text{ mm}^4$$

$$I_2 = \frac{n_2}{12} b_2 h_2^3 + n_2 A_2 d_2^2 = \frac{16.667}{12}(24)(50)^3 + (20000.4)(36)^2 = 30.09 \times 10^6 \text{ mm}^4$$

$$I_3 = \frac{n_3}{12} b_3 h_3^3 + n_3 A_3 d_3^2 = \frac{16.667}{12}(125)(12)^3 + (25000.5)(67)^2 = 112.53 \times 10^6 \text{ mm}^4$$

$$I = I_1 + I_2 + I_3 = 491.53 \times 10^6 \text{ mm}^4 = 491.53 \times 10^{-6} \text{ m}^4$$

$$M = 50 \text{ kNm}$$

$\sigma_w = -19.2 \text{ MPa}$ ◀

$\sigma_s = 123.8 \text{ MPa}$ ◀